PRIEST LAKE WATER MANAGEMENT PROJECT
OUTLET DAM IMPROVEMENTS

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COVER SHEET
INDEX OF DRAWINGS

SHEET 1
EXISTING CONDITIONS

LOCATION MAP
NOT TO SCALE

OBLIQUE AERIAL PHOTO
NOT TO SCALE

E. Sheesley
T. Morrison
S. Phillips

Priest Lake Water Management Project
Outlet Dam Improvements

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AIA DOCUMENT NO. B101-2007

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376997
1
25
CS-1
GENERAL NOTES

1. WORK DATASED ON THE DRAWINGS AND ALL APPLICABLE ITEMS OF THE SPECIFICATIONS.

2. DRAWINGS TO READ IN CONJUNCTION WITH SPECIFICATIONS AND MUSICAL INSTRUMENTS. THE CONTRACTOR'S ACTIVITY FOR THE PERIODIC REFERENCE TO CONSTRUCTION DRAWINGS WILL NOT BE PERMITTED.

3. THE CONTRACTOR IS RESPONSIBLE FOR THE VERIFICATION AND CORRECTING ANY ERRORS OR DISCREPANCIES IN THE DRAWINGS AND DETAILS PRIOR TO PROCEEDING WITH ANY WORK. IMMEDIATELY NOTIFIED OF ANY ERRORS OR DISCREPANCIES FOUND.

4. CONTRACTOR PROVIDE AND FIELD SHORING, BRACING AND OTHER DEVICES REQUIRED OR DIRECTED BY THE OWNER. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR THE OWNERS REPRESENTATIVE.

5. CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES KNOWLEDGEABLE OF THE OWNERS REPRESENTATIVE. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

6. THE CONTRACTOR SHALL PROVIDE THE CONTRACTOR'S ACTIVITIES TO ALL ACCESS ROADS TO THE OWNERS SATISFACTORY TO THE EXISTING CONDITIONS AND UTILITIES.

7. ALL DRAWINGS AND SPECIFICATIONS MAY BE REQUIRED TO COMPLETE THE WORK.

REPAIRMAN NOTES

1. THE CONTRACTOR SHALL EXAMINE AND VERIFY THAT THE FIELD CONSTRUCTION DRAWINGS AND DETAILS ARE IN ACCORDANCE WITH THE DRAWINGS AND DETAILS AND SHALL BE ADJUSTED TO REASONABLE SATISFACTION.

2. THE CONTRACTOR SHALL TAKE ALL SOIL CUT FIELD MEASUREMENTS AS REQUIRED TO VERIFY THE ACCURACY OF THE FIELD CONDITIONS AND DIMENSIONS.

3. THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR THEIR OWN WORK.

4. THE CONTRACTOR SHALL SUBMIT THE SHOP DRAWINGS TO THE OWNERS REPRESENTATIVE. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES WITHIN THE DRAWINGS SUBMITTED FOR APPROVAL.

5. THE CONTRACTOR SHALL COMPLY WITH THE LOCAL NOISE ORDINANCE.

6. THE CONTRACTOR SHALL PROVIDE THE CONTRACTOR'S ACTIVITIES TO ALL ACCESS ROADS TO THE OWNERS SATISFACTORY TO THE EXISTING CONDITIONS AND UTILITIES.

REMOVAL, EXCAVATION, AND BACKFILL NOTES

1. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

2. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

3. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

REHABILITATION NOTES

1. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

2. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

3. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

4. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

5. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

6. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

7. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

8. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

9. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

10. THE CONTRACTOR SHALL PROVIDE ALL EXISTING CONDITIONS AND UTILITIES.

ABBREVIATIONS

1. IDAHO TRANSPORTATION DEPARTMENT

2. IDAHO WATER RESOURCES BOARD

3. ACI - AMERICAN CONCRETE INSTITUTE

4. AWC - AMERICAN WELDING SOCIETY

5. APPROACH - APPROPRIATELY

6. B/D - BEGIN/DRAWING

7. B/R - BOTH WAYS

8. B:R - BOTH SIDES

9. COMM - COMMUNICATIONS

10. COF - CONCRETE FOUNDATION

11. CONC - CONCRETE

12. EQ - ELEVATION

13. F:R - FRONT SIDE

14. F:R - FRONT RAIL

15. G:R - GATE REMOVAL

16. H:R - HANGING REMOVAL

17. I:R - WITHOUT INTERFERENCE WITH OR DAMAGE TO THESE STRUCTURES, ENGINEERING TECHNICAL LETTER (ETL) 1110-2-2702, JANUARY 2000.

18. JOB:R - JOB SIDE REMOVAL

19. JOB:R - JOB SIDE REMOVAL

20. KEEP:R - KEEP SIDE REMOVAL

21. KEEP:R - KEEP SIDE REMOVAL

22. L:R - LEFT SIDE REMOVAL

23. L:R - LEFT SIDE REMOVAL

24. L:R - LEFT SIDE REMOVAL

25. M:R - MIDDLE REMOVAL

26. M:R - MIDDLE REMOVAL

27. M:R - MIDDLE REMOVAL

28. O:R - OFFICE REMOVAL

29. O:R - OFFICE REMOVAL

30. O:R - OFFICE REMOVAL

31. P:R - PROJECT SIDE REMOVAL

32. P:R - PROJECT SIDE REMOVAL

33. P:R - PROJECT SIDE REMOVAL

34. R:R - RIGHT SIDE REMOVAL

35. R:R - RIGHT SIDE REMOVAL

36. R:R - RIGHT SIDE REMOVAL

37. S:R - SIDE REMOVAL

38. S:R - SIDE REMOVAL

39. S:R - SIDE REMOVAL

40. T:R - TERRAIN SIDE REMOVAL

41. T:R - TERRAIN SIDE REMOVAL

42. T:R - TERRAIN SIDE REMOVAL

43. U:R - UPPER REMOVAL

44. U:R - UPPER REMOVAL

45. U:R - UPPER REMOVAL

46. V:R - VERTICAL REMOVAL

47. V:R - VERTICAL REMOVAL

48. V:R - VERTICAL REMOVAL

49. W:R - WITHOUT INTERFERENCE WITH OR DAMAGE TO THESE STRUCTURES, ENGINEERING TECHNICAL LETTER (ETL) 1110-2-2702, JANUARY 2000.
REINFORCING BAR EMBEDMENT/LAP SCHEDULE

<table>
<thead>
<tr>
<th>BAR SIZE</th>
<th>EMBEDMENT LAP</th>
<th>BAR EMBEDMENT LAP</th>
</tr>
</thead>
<tbody>
<tr>
<td>#3</td>
<td>20&quot;</td>
<td>20&quot;</td>
</tr>
<tr>
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<td>30&quot;</td>
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<td>42&quot;</td>
</tr>
<tr>
<td>#7</td>
<td>50&quot;</td>
<td>50&quot;</td>
</tr>
</tbody>
</table>

NOTES:
1. TOP BAR SIZES WHICH ARE ORIENTED HORIZONTALLY SHOWN ARE MORE THAN 1/4" OF CONCRETE BELOW THE BAR.
2. SPLICE BOTH BARS AT SUPPORTS.
3. SPLICE BOTH BARS AT SUPPORTS.
4. WHERE CONCRETE DEPTH EXCEEDS 5/8".
5. PROVIDE CONCRETE PROTECTION FOR CONCRETE WALLS.
6. EMBEDMENT AND LAP SCHEDULE SHOWN ARE APPROPRIATE TO ASTM A416 GR 60 OR 60/60/60 BAR.
NOTE

1. FOR LOCATION OF GROUND PHOTOS 12, 13, AND 14, SEE DWG. C-1.
DEWATERING AND CONSTRUCTION PHASING NOTES

1. STREAM DIVERSION / DEWATERING

A. DIVERSION AND CARE OF WATER IS RECOMMENDED TO OCCUR IN THE FOLLOWING ORDER; HOWEVER, THE NUMBER OF PHASES SHALL BE DETERMINED BY THE CONTRACTOR.

1. CONSTRUCT ACCESS ROAD / STRUCTURE AND SILT CURTAINS DURING ACCESS / STRUCTURE CONSTRUCTION.
   - INSTALL PHASE 1 COFFERDAMS.
   - INSTALL PHASE 1 WATER DISCHARGE CONTROL SETTLING BASIN AND PERFORM PHASE 1 AREA DRAINAGE.
   - CONSTRUCT BAYS 1 TO 5.
   - DISCONTINUE PUMPING AND ALLOW COFFERDAM AREA TO FLOOD TO THE LEVEL OF THE ADJACENT POOL.
   - INSTALL PHASE 2 COFFERDAMS AND SILT CURTAINS.
   - INSTALL PHASE 2 WATER DISCHARGE CONTROL SETTLING BASIN AND PERFORM PHASE 2 AREA DRAINAGE.
   - CONSTRUCT BAYS 6 TO 10.
   - DISCONTINUE PUMPING AND ALLOW COFFERDAM AREA TO FLOOD TO THE LEVEL OF THE ADJACENT POOL.
   - REMOVE COFFERDAMS AND RESTORE SITE.

B. THE DIVERSION SYSTEM / TEMPORARY COFFERDAM SHALL BE DESIGNED BY THE CONTRACTOR, SEE SPECIFICATIONS.

C. A TWO STAGE DEWATERING SYSTEM SHALL BE USED, SEE SPECIFICATIONS FOR WATER QUALITY REQUIREMENTS.

2. THE CONTRACTOR SHALL PROVIDE A COFFERDAM THAT WILL ALLOW FOR Dewatering AND CONSTRUCTION OF THE WORK WITHIN THE COFFERDAM LIMITS SHOWN ON THE PLAN. THE COFFERDAMS SHOWN ARE SCHEMATIC ONLY. ALL COFFERDAM DESIGNS, DETAILS, AND PLACEMENTS ARE THE RESPONSIBILITY OF THE CONTRACTOR. SEE SPECIFICATION FOR ADDITIONAL REQUIREMENTS.

3. THE CONTRACTOR SHALL HAVE A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF IDAHO PREPARE A SET OF COFFERDAM PLANS AND CALCULATIONS WHICH ARE TO BE SUBMITTED TO THE OWNERS REPRESENTATIVE FOR REVIEW AND APPROVAL. ALL PLANS AND CALCULATIONS SHALL BEAR THE SEAL AND SIGNATURE OF THE PROFESSIONAL ENGINEER LICENSED IN THE STATE OF IDAHO PREPARING THE DOCUMENTS.

4. CONTRACTOR SHALL CONSTRUCT SEDIMENT TRAPS SO THEY ARE CAPABLE OF PREVENTING TURBID WATER FROM ENTERING THE RIVER.

5. CONTRACTOR'S COFFERDAM DESIGN SHALL INCLUDE DETAILS FOR THE CONNECTIONS AT THE PER AND ABUTMENT INTERFACES TO SEAL THEM AND CONTROL SEEPAGE.

6. CONTRACTOR'S COFFERDAM DESIGN SHALL INCLUDE DETAILS FOR THE INTERFACE BETWEEN THE EXISTING STREAMBED IN PHASE 1 AND NEW STREAM BED ARMORING IN PHASE 2 TO SEAL THE COFFERDAMS AND CONTROL SEEPAGE.

7. CONTOURS SHOWN MAY NOT DEPICT ALL CONDITIONS WITHIN THE STREAMBED THAT MAY AFFECT THE DESIGN OF THE COFFERDAMS AND HEIGHT OF RETAIINED WATER LEVELS VARY.

8. CONTRACTOR SHALL CONSTRUCT A TEMPORARY SETTLEMENT BASIN FOR WATER DISCHARGE CONTROL AND PERFORM DEWATERING FOR THE DURATION OF THE WORK IN PHASE 1 & 2 OF CONSTRUCTION.

9. CONTRACTOR SHALL PROVIDE PUMPING AND MAINTENANCE OF THE WATER DISCHARGE CONTROL SETTLING BASIN FOR THE DURATION OF THE PROJECT.

10. CONTRACTOR SHALL DISCONTINUE PUMPING AND ALLOW THE COFFERDAM AREA TO FLOOD TO THE LEVEL OF THE ADJACENT POOL UPON COMPLETION AND ACCEPTANCE OF THE WORK FOR EACH PHASE REQUIRING COFFERDAMS BY OWNER'S REPRESENTATIVE.

11. COFFERDAM REMOVAL OPERATIONS SHALL NOT CAUSE AN INCREASE IN TURBIDITY IN THE RIVERBED.

12. CONTRACTOR SHALL DETERMINE THE METHOD TO REDUCE TURBIDITY DURING CONSTRUCTION TO MEET PERMIT REQUIREMENTS. SILT CURTAIN IS INDICATIVE ONLY AND SHOWN DURING COFFERDAM INSTALLATION, ACCESS STRUCTURE CONSTRUCTION, AND ELEVATION OPERATIONS.

13. HISTORICAL STREAMFLOW DISCHARGE STATISTICS (2, 5, AND 10-YR) AND CORRESPONDING WATER LEVELS FOR UNOBSERVED FLOW THROUGH 11 GATES ARE PROVIDED IN THE TECHNICAL SPECIFICATIONS AND APPENDICES. CONTRACTOR TO DETERMINE DEWATERING DESIGN. WATER LEVELS DURING CONSTRUCTION WILL VARY BASED ON STREAM DISCHARGE AND THE NUMBER OF SPILLWAY BAYS THAT ARE CLOSED OFF FOR THE PURPOSES OF THE DIVERSION; CONTRACTOR TO DETERMINE DEPTH OF FLOW AND DESIGN THE COFFERDAM AND Dewatering SYSTEMS ACCORDINGLY WITH THE SPECIFICATIONS.

14. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE WORK, PRIOR TO FINAL ACCEPTANCE. FOR FLOW CONDITIONS UP TO A 10-YR FLOW EVENT, SEE TECHNICAL SPECIFICATIONS.

TEMPORARY ACCESS ROAD/STRUCTURE NOTES

1. CONTRACTOR SHALL CONSTRUCT A TEMPORARY ACCESS ROAD/STRUCTURE FOR CONSTRUCTION ACCESS TO BAYS 6 TO 11 FOR PHASE 1 CONSTRUCTION AS NEEDED. THE CONTRACTOR SHALL CONSTRUCT A TEMPORARY ACCESS ROAD/STRUCTURE FOR STRUCTURE CONSTRUCTION.

2. CONTRACTOR SHALL CONSTRUCT A TEMPORARY SETTLING BASIN FOR WATER DISCHARGE CONTROL AND PERFORM DEWATERING FOR THE DURATION OF THE WORK FOR EACH PHASE REQUIRE COFFERDAMS BY OWNER'S REPRESENTATIVE.

3. CONTRACTOR SHALL PROVIDE PUMPING AND MAINTENANCE OF THE WATER DISCHARGE CONTROL SETTLING BASIN FOR THE DURATION OF THE PROJECT.

4. CONTRACTOR SHALL DISCONTINUE PUMPING AND ALLOW THE COFFERDAM AREA TO FLOOD TO THE LEVEL OF THE ADJACENT POOL UPON COMPLETION AND ACCEPTANCE OF THE WORK FOR EACH PHASE REQUIRING COFFERDAMS BY OWNER'S REPRESENTATIVE.

5. COFFERDAM REMOVAL OPERATIONS SHALL NOT CAUSE AN INCREASE IN TURBIDITY IN THE RIVERBED.

6. CONTRACTOR SHALL DETERMINE THE METHOD TO REDUCE TURBIDITY DURING CONSTRUCTION TO MEET PERMIT REQUIREMENTS. SILT CURTAIN IS INDICATIVE ONLY AND SHOWN DURING COFFERDAM INSTALLATION, ACCESS STRUCTURE CONSTRUCTION, AND ELEVATION OPERATIONS.

7. HISTORICAL STREAMFLOW DISCHARGE STATISTICS (2, 5, AND 10-YR) AND CORRESPONDING WATER LEVELS FOR UNOBSERVED FLOW THROUGH 11 GATES ARE PROVIDED IN THE TECHNICAL SPECIFICATIONS AND APPENDICES. CONTRACTOR TO DETERMINE DEWATERING DESIGN. WATER LEVELS DURING CONSTRUCTION WILL VARY BASED ON STREAM DISCHARGE AND THE NUMBER OF SPILLWAY BAYS THAT ARE CLOSED OFF FOR THE PURPOSES OF THE DIVERSION; CONTRACTOR TO DETERMINE DEPTH OF FLOW AND DESIGN THE COFFERDAM AND Dewatering SYSTEMS ACCORDINGLY WITH THE SPECIFICATIONS.

8. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE WORK, PRIOR TO FINAL ACCEPTANCE. FOR FLOW CONDITIONS UP TO A 10-YR FLOW EVENT, SEE TECHNICAL SPECIFICATIONS.
NOTES:
1. INSTALL TEMPORARY SEDIMENT CONTROL BARRIERS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS AND SPECIFICATIONS. THE DIMENSIONS SHOWN ARE GENERAL GUIDELINES.
2. PLACE SEDIMENT BARRIERS TO FOLLOW THE SLOPE CONTOURS. METAL Posts or wood stakes may be used.
3. ENSURE THAT RUNOFF PASSES THROUGH THE SILT FENCE AND NOT AROUND THE FENCE.
4. THE NEED FOR TEMPORARY SEDIMENT CONTROL DEVICES ARE DETERMINED BY SITE DESIGN. SPACE SILT FENCES IN ACCORDANCE WITH THE SILT FENCE SPACING TABLE.
5. EXTEND OR JOIN SILT FENCE USING SILT FENCE LAP WITH NESTED POSTS.
6. REMOVE SEDIMENT FROM THE UPLRISON SIDE OF SILT FENCES WHEN ACCUMULATION HAS REACHED THE EFFECTIVE HEIGHT OF THE BARRIER.
7. SILT CURTAIN SHALL BE A MAXIMUM OF 100 FEET LONG FOR EACH SECTION OF CURTAIN REQUIRED. END SECTIONS SHALL TERMINATE 10 FEET BEYOND THE LIMIT OF EXIT GRANUL.
8. THE SILT CURTAIN SHALL BE PLACED AS CLOSE TO THE WORK AS POSSIBLE WITHOUT INTERFERING WITH CONSTRUCTION OPERATIONS.
9. THE CONTRACTOR SHALL CONTINUALLY MONITOR THE INSTALLATION, TAKING INTO ACCOUNT WEATHER PATTERNS AND PREVAILING WIND DIRECTIONS THAT MAY AFFECT WATER LEVELS, VELOCITY AND MOVEMENT OF THE SILT CURTAIN.
10. THE SILT CURTAIN SHALL BE REMOVED BY PULLING TOWARDS THE SHORE TO MINIMIZE ESCAPE OF SEDIMENTS INTO THE WATERWAY.
11. THE WEIGHTED ANCHORING SYSTEM SHALL BE A TYPE THAT ALLOWS THE CURTAIN TO CONFORM TO THE CONTOUR OF THE BOTTOM OF THE WATERWAY.
12. CONSTRUCTION, DISTURBANCE AND LAYOUT AREAS SHOWN ON PLAN ARE APPROXIMATE AND THE CONTRACTOR IS RESPONSIBLE FOR THE COST RELATED TO CHANGES TO THE SWMP AT NO ADDITIONAL COST TO THE OWNER IF ANY OF THESE AREAS ARE EXCEEDED.
13. THE CONTRACTOR IS RESPONSIBLE TO ENSURE ALL AREAS OF SOIL DISTURBANCE ARE STABILIZED DURING CONSTRUCTION AND WHENEVER WORK IS SUSPENDED ON THE PROJECT. CONTRACTOR SHALL SEED AND MULCH THESE AREAS AS REQUESTED BY THE OWNER'S REPRESENTATIVE AT NO ADDITIONAL COST TO THE OWNER.
14. CONTRACTOR SHALL NOT WASH CONCRETE TRUCKS ONTO THE BARE GROUND, DIRECTLY INTO STORM OR SANITARY SYSTEMS INCLUDING SWALES, DITCHES OR ADJACENT PROPERTIES. EXCESS CONCRETE AND WASH WATER SHALL BE COLLECTED IN WASH BASIN AND DISPOSED OF PROPERLY.

**SILT FENCE SPACING TABLE**

<table>
<thead>
<tr>
<th>SLOPE</th>
<th>SILT TYPE</th>
<th>CLAYS</th>
<th>SANDY</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>01</td>
<td>20 ft</td>
<td>010 ft</td>
</tr>
<tr>
<td>1</td>
<td>20</td>
<td>10 ft</td>
<td>150 ft</td>
</tr>
<tr>
<td>4</td>
<td>10</td>
<td>150 ft</td>
<td>150 ft</td>
</tr>
<tr>
<td>4</td>
<td>150</td>
<td>150 ft</td>
<td>200 ft</td>
</tr>
</tbody>
</table>

**Notes:**
- Dimension variations. See drawing C.2 or as designated in the field by the owner’s representative, typ.
- Place geotextile for permanent erosion control - Class B, and a minimum of 3½’ Crushed Rock Under the Spans. From the edge of the existing roadway to the side returns, or as directed by the owner’s representative.

**Temporary Construction Entrence:**
- Place geotextile for permanent erosion control - Class B, and a minimum of 3½’ Crushed Rock Under the Spans. From the edge of the existing roadway to the side returns, or as directed by the owner’s representative.

**Temporary Guardrail:**
- Metal posts (typ.) or wood stakes may be used.
- Place sediment barriers to follow the slope contours. Metal posts or wood stakes may be used.
- Ensure that runoff passes through the silt fence and not around the fence.
- The need for temporary sediment control devices are determined by site design. Space silt fences in accordance with the silt fence spacing table.
- Extend or join silt fence using silt fence lap with nested posts.
- Remove sediment from the uplerside of silt fences when accumulation has reached the effective height of the barrier.
- The silt curtain shall be a maximum of 100 feet long for each section of curtain required. End sections shall terminate 10 feet beyond the limit of exit granule.
- The silt curtain shall be placed as close to the work as possible without interfering with construction operations.
- The contractor shall continually monitor the installation, taking into account weather patterns and prevailing wind directions that may affect water levels, velocity and movement of the silt curtain.
- The silt curtain shall be removed by pulling toward the shore to minimize escape of sediments into the waterway.
- The weighted anchoring system shall be a type that allows the curtain to conform to the contour of the bottom of the waterway.
- Construction, disturbance and layout areas shown on plan are approximate and the contractor is responsible for the cost related to changes to the SWMP at no additional cost to the owner if any of these areas are exceeded.
- The contractor is responsible to ensure all areas of soil disturbance are stabilized during construction and whenever work is suspended on the project. Contractor shall seed and mulch these areas as requested by the owner’s representative at no additional cost to the owner.
- Contractor shall not wash concrete trucks onto the bare ground, directly into storm or sanitary systems including swales, ditches or adjacent properties. Excess concrete and wash water shall be collected in wash basins and disposed of properly.
PROPOSED SITE PLAN

NOTES

1. CABLES FOR VWP CABLING SHOULD EXTEND FROM VWP CONTROL PANEL AT END OF DRAIN TO VWP CONTROL PANEL. VWP CABLES SHOULD BE STRIPPED BARE AT TIES AND PREPARED FOR TIES. CABLES SHOULD BE PROTECTED FROM WEATHER. WIRES SHALL BE LABELED TO KNOW WHICH VWP IS WHICH.

2. CONFIGURATION OF THE CONDUIT IS AT CONTRACTOR'S OPTION. VWP CABLES SHOULD BE ORDERED OF SUFFICIENT LENGTH TO COMPLETE THE RUN.

3. ORDINARY HIGH WATER (OHW) LIMITS OF CONSTRUCTION PROPERTY LINES

LEGEND

- ARMOR STONE
- CONCRETE APRON EXTENSION
- PROPERTY LINES
- ORDINARY HIGH WATER (OHW)
- LIMITS OF CONSTRUCTION

ELEVATION

BOTTOM OF KEYWAY

APRON WALL

EXISTING WINDOW WALL

EXISTING OUTLET STRUCTURE

VWP CONTROL PANEL

ELECTRIC VIBRATING WIRE PIEMETER (VWP 02), TYP. (2 TOTAL)

CONCRETE APRON EXTENSION

EXISTING APRON WALL

EXISTING WINGWALL

PROPOSED SITE PLAN

SCALE: 1" = 10'

DRAFT FOR BID

Priest Lake Water Management Project
Outlet Dam Improvements
GENERAL DAM PLAN AND ELEVATION

376997 12 25 As Noted S-1
WORK LIST
MISCELLANEOUS ELEMENTS
1. REPLACE DAMAGED PORTION OF HANDRAIL
2. INSTALL VIBRATING WIRE PIEZOMETERS AT 3 LOCATIONS W/ CONTROL PANEL ON N. ABUTMENT.

NOTES
1. SHEET FILE WALL IS SHOWN PER 1978 DESIGN DRAWINGS.
2. DOWNSTREAM ARMOR STONE AND BEDDING STONE UNDER LAYER LIMITS SHOWN PER TECHNICAL SECTION DESIGN DRAWINGS.
3. SLOPE OF ARMOR STONE SURFACE VARIES TO MEET EXISTING
4. THE LIMITS OF ORIGINAL ARMOR STONE SHOWN IN SECTION A EXTEND ACROSS THE ENTIRE WIDTH OF THE DAM ALONG THE DOWNSTREAM EDGE. LIMITS IN THE SECTION ARE BASED ON 1978 DESIGN DRAWINGS AND SITE CONDITIONS OBSERVED IN THE FIELD.
5. DUE TO THE EXISTING STREAM BED MATERIALS, COMPACTED NATIVE MATERIALS UNDER LAYER EFFECTS OF RIVER FLOW THE CONTRACTOR CAN ANTICIPATE ENCOUNTERING ARMOR STONE, COBBLES, Boulders, AND OTHER OBSTRUCTIONS IN THE RIVERBED AND BANKS OF THE PROPOSED CONSTRUCTION ZONE. THE COST TO REMOVE, DISPOSE OF, STORE, OR BENEFIT THE ARMOR STONE, COBBLES, Boulders, AND OTHER OBSTRUCTIONS TO FACILITATE SAFE CONSTRUCTION SHALL BE AT CONTRACTOR'S EXPENSE.
6. THE LIMITS OF DISPLACED ARMOR STONE SHOWN ARE APPROXIMATE. ACTUAL WIDTHS AND DEPTHS MAY VARY. CONTRACTOR SHALL VERIFY THE CONDITIONS IN THE FIELD.
7. VWP SHALL BE EQUVALENT TO DURHAM GEO SLOPE INDICATOR PART NUMBER 8200, RATED STEEL, VARIES WITH CONTRACTOR'S DESIGN. BUT ITS ELEVATION SHALL BE MEASURED TO BE ACCURATE TO WITHIN 2 INCHES.
8. VWP SHALL BE LOCATED IN A BOREHOLE EMBEDDED BETWEEN 10 AND 15 FEET BELOW THE APRON AT CONTRACTOR'S DESIGNATION. BUT ITS ELEVATION SHALL BE MEASURED TO BE ACCURATE TO WITHIN 2 INCHES.
9. CONTRACTOR TO PROVIDE AS-BUILT DRAWING OR MARKUP IDENTIFYING ELEVATION OF VWP, CONTROL PANEL, AND VWP LABELING AT VWP CONTROL PANEL.

TYPICAL EXISTING SECTION

TYPICAL PROPOSED SECTION

OUTLET STRUCTURE
SECTIONS AND WORK LIST

Priest Lake Water Management Project
Outlet Dam Improvements

DEPARTMENT OF WATER RESOURCES
1601 5th Avenue
Seattle, Washington 98101

IDAH0

RCF: 22070

DRAFT FOR BID

OUTLET STRUCTURE

MOTT MACDONALD

301 5th Avenue
Seattle, Washington 98121

T: 1-800-779-6883
F: 206-247-6500
M: mottmac.com

PROJECT MANAGER

J. Dawson

S. Phillips

DEPARTMENT MANAGER

OUTLET STRUCTURE

MOTT MACDONALD

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Drawn

Date

25

376997

SECTIONS AND WORK LIST

SP

SK

TM

90% SUBMITTAL

OUTLET STRUCTURE

Priest Lake Water

Management Project

Outlet Dam Improvements

DEPARTMENT OF WATER RESOURCES
1601 5th Avenue
Seattle, Washington 98101

IDAH0

RCF: 22070

DRAFT FOR BID

OUTLET STRUCTURE

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PROJECT MANAGER

J. Dawson

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DEPARTMENT MANAGER

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NOTES

1. Tainter gate extension plates shall be continuous over the full width of the existing gates. The extensions shall be accurately fitted and bolt holes drilled in the shop for proper installation. Field drilling of bolt holes in extensions not allowed. New holes in existing tainter gate skin plates shall be field drilled using the existing tainter gate skin plate templates. The extensions shall be fitted using the existing tainter gate skin plate dimensions and radii prior to fabrication and drill of fabrication dimensions based on these measurements and original shop drawings in order to prepare the extension shop drawings.

2. Tainter gate strengthening components shall be accurately fitted and bolt holes drilled in the shop for proper installation. Field drilling of bolt holes in strengthening components not allowed. Bolt holes in existing tainter gate skin plates and framing shall be field drilled using the strengthening components as a prepared template to ensure proper fitting of pieces. Contractor shall field measure the existing tainter gate skin plate and framing dimensions including radii prior to fabrication and drill of fabrication dimensions based on these measurements and original shop drawings in order to prepare the extension shop drawings.

3. Contractor shall provide temporary lifting/shoring to support tainter gates during trunnion pin replacement. The contractor shall employ suitable equipment and methods so as to not damage or overstress any members in the tainter gates. All members in the tainter gate assembly shall be safely braced and firmly held in place until they can be re-supported on the new trunnion pins.

4. The contractor shall prepare a set of detail plans and calculations for the structural lifting/shoring operations. These plans and calculations shall be stamped by an Idaho State License Professional Engineer. The plans shall include a detailed outline of the structural lifting/shoring procedure. The structural lifting system shall be designed in accordance with AASHTO LRFD Bridge Specifications.

5. Structural lifting/shoring of tainter gates shall only occur when Cofferdam is in place and in area that is dewatered. Structural lifting/shoring of tainter gates shall not be undertaken under any circumstances on gates that are still operational, or in an area that is not dewatered outside the Cofferdam.

6. The contractor's structural lifting and shoring plan shall include all details and design checks required to ensure the existing substructure remains stable.

7. The contractor shall prepare a set of detail plans and calculations for the structural lifting/shoring operations. The plans and calculations shall be stamped by an Idaho State License Professional Engineer. The plans shall include a detailed outline of the structural lifting/shoring procedure. The structural lifting system shall be designed in accordance with AASHTO LRFD Bridge Specifications. The contractor shall provide temporary lifting/shoring to support tainter gates during trunnion pin replacement. The contractor shall employ suitable equipment and methods so as to not damage or overstress any members in the tainter gates. All members in the tainter gate assembly shall be safely braced and firmly held in place until they can be re-supported on the new trunnion pins.

8. Contractor shall provide temporary lifting/shoring to support tainter gates during trunnion pin replacement. The contractor shall employ suitable equipment and methods so as to not damage or overstress any members in the tainter gates. All members in the tainter gate assembly shall be safely braced and firmly held in place until they can be re-supported on the new trunnion pins.

9. The structural lifting/shoring of tainter gates shall only occur when Cofferdam is in place and in area that is dewatered. Structural lifting/shoring of tainter gates shall not be undertaken under any circumstances on gates that are still operational, or in an area that is not dewatered outside the Cofferdam.

10. The contractor's structural lifting and shoring plan shall include all details and design checks required to ensure the existing substructure remains stable.

"SCALE: 3/4" = 1'-0"

SECTION

ELEVATION

SCALE: 1/4" = 1'-0"

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Priest Lake Water Management Project
Outlet Dam Improvements
TAINTER GATE ELEVATION AND SECTION

As Noted

376997 14 25 S-3
NOTES
1. REMOVE AND REPLACE EXISTING LUBE LINE AT BOTH TRUNNION PINS ON ALL 11 GATES WITH A 1" SAE J525 ANNEALED STEEL SEAMLESS TUBE LUBE LINE RATED FOR A MINIMUM 5000 PSI PRESSURE.
2. CONTRACTOR SHALL SUPPLY OWNER WITH (2) LINCOLN 1882 20V POWERLUBER HANDHELD BATTERY OPERATED GREASE GUN RATED FOR A MAXIMUM GREASE DELIVERY PRESSURE UP TO 10000 PSI.
3. CONTRACTOR IS RESPONSIBLE FOR SUPPLYING ALL LUBE LINES, FITTINGS, AND BRACKETS TO HOLD LUBE LINE IN PLACE ON CONCRETE PIER/ABUTMENT.

GREASE FITTING REPAIR DETAIL
NOT TO SCALE

EXISTING TRUNNION ARM
EXISTING CONCRETE PIER
EXISTING CONCRETE WALKWAY
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING CONCRETE WALKWAY
EXISTING HANDRAIL
EXISTING HANDRAIL
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING CONCRETE WALKWAY
EXISTING CONCRETE PIER
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING TRUNNION ARM
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING CONCRETE PIER
EXISTING CONCRETE WALKWAY
EXISTING HANDRAIL
EXISTING HANDRAIL
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING CONCRETE WALKWAY
EXISTING CONCRETE PIER
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING CONCRETE PIER
EXISTING CONCRETE WALKWAY
EXISTING HANDRAIL
EXISTING HANDRAIL
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING CONCRETE WALKWAY
EXISTING CONCRETE PIER
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING CONCRETE PIER
EXISTING CONCRETE WALKWAY
EXISTING HANDRAIL
EXISTING HANDRAIL
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING CONCRETE WALKWAY
EXISTING CONCRETE PIER
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING CONCRETE PIER
EXISTING CONCRETE WALKWAY
EXISTING HANDRAIL
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EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
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(SEE NOTE 1)
EXISTING CONCRETE PIER
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EXISTING HANDRAIL
EXISTING HANDRAIL
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING CONCRETE WALKWAY
EXISTING CONCRETE PIER
EXISTING WORM GEAR ASSEMBLY AND DRIVE SHAFT
EXISTING TRUNNION LUBE LINE, BRACKETS AND FITTINGS - TYPICAL
(SEE NOTE 1)
EXISTING UPSTREAM HAND RAIL REPLACEMENT DETAIL

PROPOSED UPSTREAM HAND RAIL REPLACEMENT DETAIL

EXISTING UPSTREAM RAILING

PROPOSED HSS 2X2X11

DETAIL

SCALE: 1" = 1/4"
NOTES:

1. CONTRACTOR SHALL PLACE CONCRETE APRON EXTENSION WITH APRON WALLS, UPSTAND, KEYWAY, AND FLOATING SLAB TO LIMITS IDENTIFIED. SEE DRAWINGS A-3 THRU A-5 FOR SECTIONS AND DETAILS.

2. CONTRACTOR SHALL PLACE ARMOR STONE DOWNSTREAM OF APRON EXTENSION EDGE TO LIMITS IDENTIFIED. SEE DRAWINGS A-2 FOR PLACEMENT DETAIL.

3. CONTRACTOR SHALL SEAL EXISTING CONTRACTION JOINTS AT THE 3 IDENTIFIED LOCATIONS ON THE PLAN. SEE DRAWING M-1 FOR DETAIL.

4. CONTRACTOR SHALL SEAL EXISTING CONTRACTION JOINTS AT THE 3 IDENTIFIED LOCATIONS ON THE PLAN. SEE DRAWING M-1 FOR DETAIL.

5. CONTRACTOR SHALL SEAL EXISTING CONTRACTION JOINTS AT THE 3 IDENTIFIED LOCATIONS ON THE PLAN. SEE DRAWING M-1 FOR DETAIL.

6. CONTRACTOR SHALL SEAL EXISTING CONTRACTION JOINTS AT THE 3 IDENTIFIED LOCATIONS ON THE PLAN. SEE DRAWING M-1 FOR DETAIL.

7. CONTRACTOR SHALL SEAL EXISTING CONTRACTION JOINTS AT THE 3 IDENTIFIED LOCATIONS ON THE PLAN. SEE DRAWING M-1 FOR DETAIL.

8. CONTRACTOR TO LOCATE CONSTRUCTION JOINTS IN APRON EXTENSION AS INDICATED ON PLAN WITH CJ. SEE DRAWING A-5 FOR DETAIL.

9. CONTRACTOR TO REMOVE AND REPLACE EXISTING STREAM GAUGE ON THE NORTH ABUTMENT AND INSTALL A NEW STREAM GAUGE ON THE SOUTH ABUTMENT. SEE DRAWING M-1 FOR DETAIL.

10. CONTRACTOR TO LOCATE CONSTRUCTION JOINTS IN APRON EXTENSION AS INDICATED ON PLAN WITH CJ. SEE DRAWING A-5 FOR DETAIL.
NOTES:
1. EXISTING STREAMBED SHOWN IS NOT REPRESENTATIVE OF ACTUAL GRADE. IT IS EXAGGERATED TO ILLUSTRATE POTENTIAL FIELD CONDITIONS THAT COULD BE ENCOUNTERED. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THESE CONDITIONS IN THE FIELD.
2. EXISTING STREAMBED EXCAVATION DEPTH PROVIDED AS AN APPROXIMATE TO FACILITATE CONTRACTORS. SP ELEVATIONS OF EXISTING STREAMBED CAN BE FOUND ON DRAWINGS C-2 & C-3. TO PROVIDE THE CONTRACTOR A MORE COMPREHENSIVE DEPICTION OF EXISTING STREAMBED ELEVATION VARIATIONS, CONTRACTOR IS RESPONSIBLE FOR ESTIMATING QUANTITIES OF EXCAVATION AND FILL THAT MAY BE REQUIRED. CONTRACTOR SHALL BE RESPONSIBLE FOR RESPONSIBLE FOR VERIFYING GRADE VARIATIONS IN THE FIELD AND VERIFYING CONDITIONS IDENTIFIED ON PLAN AND IN SECTIONS. CONTRACTOR SHALL TAKE ADDITIONAL EXCAVATION OR FILL THAT MAY BE NEEDED DURING CONSTRUCTION AT NO ADDITIONAL COST TO THE OWNER.
3. EXISTING STREAMBED MATERIAL TO BE EXCAVATED TO THE TOP OF BEDDING LINE. IF EXCESS MATERIAL MEETS THE REQUIREMENTS OF ARMOR STONE OR STREAMBED FILL MATERIAL, CONTRACTOR SHALL RELOCATE AND RESET MATERIAL TO FILL DEEPER VOIDS BELOW THE TOP OF BEDDING LINE OR SCOUR HOLES AS DIRECTED BY OWNER'S REPRESENTATIVE. SUITABILITY OF MATERIAL TO BE DETERMINED BY OWNER'S REPRESENTATIVE. UNFITTED FOR RE-USE CONTRACTOR SHALL DISPOSE OF MATERIAL AT NO ADDITIONAL COST TO THE OWNER. SEE SPECIFICATION SECTION FOR ADDITIONAL INFORMATION.
4. INSTALL CLASS C NONWOVEN GEOSYNTHETIC FABRIC WITH MODERATE SURVIVABILITY BETWEEN NATIVE SUBGRADE AND BEDDING STONE FOR UNDERGROUND OIL AND GAS TREATMENT.
5. PLACE BEDDING STONE DIRECTLY BENEATH APRON EXTENSION AND ARMOR ROCK TO LIMITS IDENTIFIED ON PLAN AND IN SECTION. AGGREGATES USED SHALL BE FREE OF ADHERENT MATERIALS AND DELETERIOUS MATERIALS SUCH AS ORGANICS OR CONSTRUCTION MATERIALS.
6. PLACE ARMOR STONE IN STREAMBED TO TOP OF PROPOSED CONCRETE UPSTAND FOR LIMITS IDENTIFIED. ARMOR STONE MATERIAL AND GRAVATION SHALL MEET THE REQUIREMENTS LISTED IN SPECIFICATION SECTION 353123 ARMOR STONE.
7. PLACE STREAMBED MATERIAL IN STREAMBED TO MEET EXISTING GRADE. MATERIAL MEETS THE REQUIREMENTS OF ARMOR STONE OR STREAMBED FILL MATERIAL.
8. INSTALL CLASS C NONWOVEN GEOSYNTHETIC FABRIC WITH MODERATE SURVIVABILITY BETWEEN NATIVE SUBGRADE AND BEDDING STONE FOR UNDERGROUND OIL AND GAS TREATMENT. CONTRACTOR SHALL RELOCATE AND RESET MATERIAL TO FILL DEEPER VOIDS BELOW THE TOP OF BEDDING LINE OR SCOUR HOLES AS DIRECTED BY OWNER'S REPRESENTATIVE. SUITABILITY OF MATERIAL TO BE DETERMINED BY OWNER'S REPRESENTATIVE. IF OWNER'S REPRESENTATIVE DETERMINES MATERIAL IS UNSUITABLE FOR RE-USE CONTRACTOR SHALL DISPOSE OF MATERIAL AT NO ADDITIONAL COST TO THE OWNER. SEE SPECIFICATION SECTION FOR ADDITIONAL INFORMATION.
APRON REINFORCING PLAN

POUR 5 AND 6 SHOWN. POUR 4 SIMILAR TO POUR 5 AND POURS 1, 2, 3, 7 AND 8 SIMILAR TO POUR 6

POUR 5 AND 6 SHOWN. POUR 4 SIMILAR TO POUR 5 AND POURS 1, 2, 3, 7 AND 8 SIMILAR TO POUR 6
1. Removal of structural concrete shall be a minimum of 4" or to sound concrete. If poor condition concrete extends beyond the 6" removal limit, the contractor shall notify the owner's representative before removing additional concrete to reach sound concrete.

2. Contractor to maintain and protect existing reinforcement.

3. Saw cut horizontally the existing wall as dimensioned on the existing elevation; this sheet. Saw cut line shall be cut level and parallel with existing pier profile.

4. Remove the loose and unsound concrete on the pier face within the limits identified in elevation.

5. Mechanically prepare concrete surface to ensure bonding of repair material.

6. Prepare and apply bonding agent to roughened concrete surface within limits of partial depth repair identified on wall elevation in accordance with the bonding agent manufacturer's specifications.

7. Form and pour repair concrete within the limits of the partial depth repair identified in elevation.

EXISTING CONCRETE PIER

EXISTING CONCRETE APRON

EXISTING REINFORCEMENT TO REMAIN - SEE NOTE 2

EXISTING REINFORCEMENT EXPOSED AND DAMAGED DURING CONCRETE REMOVAL

NEW #5 REINF. DRILLED AND GROUTED INTO EXISTING CONCRETE MIN. OF 30" BELOW ANY NEW OR EXISTING CONSTRUCTION JOINT

FORM AND FOUR NEW CONCRETE - SEE NOTE 1

EXISTING REINFORCEMENT CLEANED AND MAINTAINED IN PLACE

LIMITS OF ORIGINAL CONCRETE REMOVAL

LIMITS OF ORIGINAL CONCRETE REMOVAL

EXISTING REINF. BROKEN DURING CONCRETE REMOVAL

NEW #5 REINF. EXPOSED AND MORTARIZED INTO EXISTING CONCRETE MIN. OF 2" BELOW ANY NEW OR EXISTING CONSTRUCTION JOINT

CONCRETE REPAIR DETAIL - 1

REINF. EXPOSED, NO DAMAGE

REINF. EXPOSED AND DAMAGED NOT TO SCALE

CONCRETE REPAIR DETAIL - 2

5'-0" 9'-0"

NEW CONCRETE - SEE NOTE 7

APPLICATION AND ANTICORROSION AGENT TO SUBSTRATE - SEE NOTE 6

EXISTING REINF. CLEANED AND MAINTAINED IN PLACE

LIMITS OF ORIGINAL CONCRETE REMOVAL

EXISTING REINF. BROKEN DURING CONCRETE REMOVAL

EXISTING REINF. EXPOSED AND MORTARIZED INTO EXISTING CONCRETE MIN. OF 2" BELOW ANY NEW OR EXISTING CONSTRUCTION JOINT

FORM AND FOUR NEW CONCRETE - SEE NOTE 1

NEW #5 REINF. EXPOSED, NO DAMAGE NOT TO SCALE
EXPANSION JOINT REPAIR PROCEDURE

1. REMOVE EXISTING JOINT SEAL OR MASTIC TO A MINIMUM DEPTH OF 2".
2. INSTALL BACKER ROD AND JOINT SEAL AS PER MANUFACTURERS RECOMMENDATIONS.
4. PERFORM SEALING ON BOTH THE DOWNSTREAM WINGWALL EXPANSION JOINT (SHOWN) AND THE UPSTREAM WINGWALL EXPANSION JOINT (NOT SHOWN).
5. INSTALL PROPOSED HYDROPHYLIC SEALER. USE Sika Hydrotite or Approved Equal as per manufacturers recommended ENS.

STREAM GAUGE NOTES

1. STREAM GAUGE SHALL BE CONSTRUCTED OF A DURABLE FIBERGLASS COMPOSITE TO ENSURE IT WILL NOT BE DAMAGED DUE TO IMPACT, ROT, OR RUST.
2. CONTRACTOR SHALL ATTACH GAUGE TO UPSTREAM FACE WITH STAINLESS STEEL ANCHORS.
3. STREAM GAUGE SHALL BE COATED WITH A NON-GLARE COATING.
1. Construction Approach in accordance with ITD standard plans.
2. Coordinate grades with adjacent existing approaches and roadways.
3. Provide granular subbase and aggregate base course on compacted subgrade in accordance with ITD specification sections 301 and 303, respectively.
4. Provide HMA road mix pavement in accordance with ITD specification section 406.

NOTES

1. Construction Approach in accordance with ITD standard plans.
2. Coordinate grades with adjacent existing approaches and roadways.
3. Provide granular subbase and aggregate base course on compacted subgrade in accordance with ITD specification sections 301 and 303, respectively.
4. Provide HMA road mix pavement in accordance with ITD specification section 406.